78 Pomme De Terre River at U.S. 12 near Holloway, MN

Site Location:

Site ID: 78

Site Name: Pomme De Terre River at U.S. 12 near Holloway, MN

County: Swift

Nearest City: Holloway Contact:

David Mueller
State: MN U.S. Geological Survey

Latitude: 9818 Bluegrass Parkway
Louisville, KY 40299

Longitude: 0955845

USGS Station ID:

Service Level:

Route Number: 12

Route Class: US Publication:

Mueller, D.S., and Hitchcock,

contracted highway crossings in Minnesota, 1997: ASCE, Water

Route Direction: NA Minnesota, 1997: ASCE, Water Resources Engineering '98, Memphis, TN, p. 210-215.

Highway Mile Point:

Stream Name: Pomme De Terre River

Mainline

River Mile:

Site Description:

Bridge 5359 is located 10.7 miles west of Danvers, Minnesota on U.S. Route 12 over the Pomme De Terre River. The single-span steel-truss structure was constructed in 1933 with a maximum span length of 88.3 ft. The upstream floodplain consists of a mixture of open agricultural land with scattered trees and brush. There is a park on the upstream left bank. The area downstream is more heavily wooded and is classified on the maps as a wetland area.

The bridge has vertical-wall abutments with wing walls. Each abutment and wing wall rest on concrete footings supported on timber piling. Neither abutment is riprapped nor do they have any other scour protection measures. A field investigation conducted by BRW, Inc. (during a scour evaluation) revealed no evidence of significant scour at the abutment face.

Regarding bed material, BRW reported the following:

The stream bed material in the vicinity of the bridge generally consists of fine grained organic silty sand. A sieve analysis of a field sample indicated a mean diameter of 0.00049 ft. Based on the soil borings and blow counts documented for the bridge construction, the river bed materials appear to become harder and denser as depth increases. Since the type of the soil was not recorded, it is difficult to ascertain the makeup of the soils at depth.

BRW noted minimal debris during their field investigation and the USGS note no

H.A., 1998, Scour measurements at

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debris during the flood measurements. There are trees locate on the upstream floodplain so the potential for debris exists, but is probably low.

The nearest gaging station is located at Appleton, MN approximately 15 miles downstream. Station No. 05293960 has over 60 years of record. There is one small drainage noted between the site and Appleton on the right floodplain.

Summary of flood frequencies from BRW's scour analysis:

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Return Period - Discharge (cfs)

10 yr - 2,075

50 yr - 3,925

100 yr - 4,880

390 yr - 7,130 overtopping flood

500 yr - 7,530
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A scour-monitoring plan had been implemented by MNDOT and was available at the time of the flood. This bridge was scheduled to be replaced and has now been replaced with a new structure.

The stream data portion of the database only allows one set of Manning's n values. Below is the estimated upstream and downstream values.

Upstream	Left	Main	Right	Downstream	Left	Main	Right
High	0.08	0.035	0.08		0.12	0.035	0.12
Typical		0.030			0.08	0.030	0.08
Low	0.05		0.05				

Elevation Reference

Datum: MSL

MSL (ft):

Description of Reference Elevation:

Elevations are referenced to MSL based on values provided by MNDOT on their scour monitoring plan. Plans for the new bridge developed by BRW showed elevations 30 ft higher. The scour report from BRW agreed with the MNDOT scour monitoring plan and thus, that elevation reference was used. The top of curb near the east abutment was used and was to have an elevation of 998.7 ft. The horizontal stationing of data collected from the bridge deck was referenced to the left abutment then adjusted in post-processing to be consistent with stationing used in the BRW WSPRO model. Distance of ADCP data from the bridge was visually estimated. Horizontal stationing for the ADCP is based on bottom tracking. The stationing was visually adjusted to agree with the BRW WSPRO model.

(3/8/2000) Note: The elevations that were provided by MNDOT, and the elevations from the BRW sour report, when used to build a HEC-RAS model of the bridge section, were discovered to be inconsistent with the downstream gaging station (Appleton) elevations during the 1997 flood. MNDOT was again contacted and it was discovered that elevation 995 ft above MSL on the BRW scour report should actually be 1023.9 feet above MSL, thus validating the new bridge plan elevations. Therefore, the elevation of the top of curb near the east abutment should actually be 1027.6 ft, making the bridge section more consistent with elevations upstream at CR 22 bridge (see entry 73) and downstream at the Appleton gaging station. A correction of +28.9 ft should be made to MNDOT's reference elevation on their sour monitoring plan and all elevations from the BRW sour report. The April 1997 field data, found in the attached excel file (us12pdt-REV.xls), has already been corrected to reflect the new reference elevation.

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Stream Data

Drainage Area 845 Floodplain Width: Wide

(sq mi):

Slope in 0.0005 Natural Levees: Unknown

Vicinity(ft/ft):

Flow Impact: Right Apparent Incision: None

Channel Evolution Premodified Channel Boundary: Alluvial

Armoring: Unknown Banks Tree Cover: Medium

Debris Frequency: Rare Sinuosity: Straight

Debris Effect: None Braiding: None

Stream Size: Small Anabranching: None

Flow Habit: Perennial Bars: Unknown

Bed Material: Sand Stream Width Equiwidth

Variability:

Valley Setting: Low

Roughness Data

Manning's n Values

	Left Overbank	Channel	Right Overbank
High:	0.08	0.035	0.08
Typical		0.03	
Low:	0.05		0.05

Bed Material

Measurement Number	Yr	Мо	Dy	Sampler		D84 (mm)	D50 (mm)	D16 (mm)	SP	Shape	Cohesion	
1	1995	7	10	IInknown	0 28	0.2	0 15		2 65		Mildly	

Bed Material Comments

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Measurement No: 1

Bed-material size distribution reported are from information provided the MnDOT. A review of the lithologic logs for the replacement bridge show that the subsurface material is primarily sands, silts, with some gravel. D16 was less than 0.062

Bridge Data

Structure No: 5359

Length(ft): 88.3

Width(ft): 27

Number of Spans: 1

Vertical Configuration: Sloping

Low Chord Elev (ft): 1023.85

Upper Chord Elev (ft): 1024.76

Overtopping Elev (ft): 1027.6

Skew (degrees): 0

Guide Banks: None

Waterway Classification: Main

Year Built: 1933

Avg Daily Traffic:

Plans on File: Yes

Parallel Bridges No

 $\label{lownstream} \textbf{Upstream/Downstream:} \quad \mathbb{N}/\mathbb{A}$

Continuous Abutment: No

Distance Between Centerlines:

Distance Between Pier Faces:

Bridge Description:

The bridge is an old truss bridge. The bridge and its approach embankments are perpendicular to the main channel. However, during the 1997 flood there was considerable skew as a significant amount of flow was coming from the left floodplain. The flow through the bridge opening in the center of the channel was skewed about 50 degrees.

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Abutment Data

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Left Station:
                 956
               1044
Right Station:
Left Skew (deg): 0
Right Skew (deg) 0
Left Abutment Length (ft): 33
Right Abutment Length (ft) 33
Left Abutment to Channel Bank (ft): 0
Right Abutment to Channel Bank (ft): 0
Left Abutment Protection:
Right Abutment Protection
Contracted Opening Type:
                           IV
Embankment Skew (deg):
                           0
Embankment Slope (ft/ft):
Abutment Slope (ft/ft)
                           0
Wingwalls:
                           Yes
Wingwall Angle (deg):
                           45
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Pier Data

Pier Scour Data

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Abutment Scour

Measurement Number	Abutment	Date	Time	US/DS	Scour Depth (Accuracy	Sediment Transport
1	Right	4/9/9	7 16:00	Upstream	8		2	Live-bed
2	Right	4/9/9	7 14:00	Upstream	7		2	Live-bed
3	Right	4/9/9	7 14:00	ownstrea	11		2	Live-bed
4	Left	4/9/9	7 16:00	Upstream	3		2	Live-bed
5	Left	4/9/9	7 14:00	Upstream	1.5		2	Live-bed
6	Left	4/9/9	7 14:00	ownstream	6		2	Live-bed
Measurement Number	Velocity Abut(f	_	Depth at Abut(ft)		_		Telocity	Avg Depth Blocked(ft)
1	4.2		30					
2	4.2		31					
3	4.2		27					
4	3.8		25					
5	3.8		25					
6	3.8		22					
Measurement Number	Embankr Length		Bed Mate	rial D50	(mm.) S	igma	Debris B	Effect
1	100	6	Unknow	m 0.	15	1.5	Insigni	ficant
2	100	6	Unknow	m 0.	15	1.5	Insigni	ficant
3	100	6	Unknow	m 0.	15	1.5	Insigni	ficant
4	396	5	Unknow	m 0.	15	1.5	Insigni	ficant
5	396	5	Unknow	m 0.	15	1.5	Insigni	ficant
6	396	5	Unknow	m 0.	15	1.5	Insigni	ficant
Abutment S	cour Com	ments						

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MeasurementNo

1

The reference surface used to determine the depth of abutment scour was the concurrent ambient bed. Therefore, the depth of abutment scour reported is additional local scour below the depth of contraction scour. For this site, it appears that the scour holes may interact as there is only one or two depth measurement between the holes that define the ambient bed.

There was significant flow from the left upstream flood plain through the bridge opening. This flow from the left flood plain significantly skewed the flow through the bridge opening, about 50 degrees on the average. Refer to maps and sketches included in Other Detailed Data.

Measurements numbers 1 and 4 were made during a discharge measurement along the upstream face of the bridge. The depths were measured with a sounding weight. All other measurements were made using an echo sounded mounted on a knee-board. The board was floated from upstream to downstream under the bridge. The measurements reflect the depths at the upstream or downstream face of the bridge.

All hydraulic information is from the one discharge measurement.

The velocity at the abutments was much higher on 4/5/97: Left - 5.1 ft /sec Right - 5.9 ft/sec

Embankment lengths were determined by projecting the measured water surface elevation onto the approach section from the BRW WSPRO model.

MeasurementNo

See comments on measurement no. 1.

MeasurementNo 6

See comments on measurement no. 1.

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ContractionScour

Contraction	oooai				
Measurement Number	Contracted C	ontracted Unc Time	ontracted Uncor Date	ntracted Time US/DS	Scour Depth(ft)
1	4/5/97	11:30	4/5/97		3.1
2	4/9/97	16:00	4/5/97		10.5
3	4/9/97	14:00	4/5/97		12.5
4	4/9/97	14:00	4/5/97		4.5
Measurement Number	Accuracy	Contracted Avg Vel(ft/s	Contracted) Discharge(ci		Contracted Width(ft)
1	2	4.8	5000	12.1	88
2	2	2.7	5750	24	88
3	2	2.8	5750	23.6	88
4	2	3.8	5750	17.3	88
Measurement Number	Uncontracted Avg Vel(ft/s)	Uncontracte Discharge(c		ed Uncontracted) Width(ft)	Channel Contraction Ratio
1	3.4	1800	7.9	70	0.64
2					
3					
4					
Measurement Number	Pier Contraction Ratio		ccent- Sedime		Debris Effects
1	M	ain Channel	Live-	oed Unknown	Unknown
2	M	ain Channel	Live-	oed Unknown	Unknown
3	M	ain Channel	Live-	Unknown	
4	M	ain Channel	Live-	oed Unknown	Unknown
Measurement Number	D95 (mm) D8	4 (mm) D50 (mm) D16 (mm)	Bed	ed erial

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1	0.28	0.23	0.15	1.5	Mildly	
2	0.28	0.23	0.15	1.5	Mildly	
3	0.28	0.23	0.15	1.5	Mildly	
4	0.28	0.23	0.15	1.5	Mildly	
Contractio	on Scour (Comments				

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Measurement No. 1

Contraction scour was computed as the difference in average bed elevation between uncontracted and contracted sections, adjusted for bed slope.

The appropriate reference surface was determined from an analysis of cross sections collected by BRW on 6/5/95 and the USGS during the flood on 4/5/97. Cross sections on these two dates collected approximately 300 ft upstream from the bridge show only about 0.5 difference the channel bottom elevation. The flood section was the lower of the two. Downstream from the bridge the cross section surveyed on 6/5/95 (approximately 75 ft downstream) and the cross section surveyed on 4/5/97 (approximately 200 ft downstream) are similar, with less than 1 ft in variation in the channel bottom elevations. The 4/5/97cross section 100 ft downstream was about 1.5 below the 6/5/97 cross section at 75 ft downstream. It was assumed that the 4/5/97 cross section could have been effected by the scour at the bridge section. Thus, it was not considered in the setting of the reference surface. The WSPRO bridge section surveyed by BRW on 6/5/95 showed from 1 to 2 ft of abutment scour in the cross-section. However, the center of the channel at the bridge appears to be representative of consistent channel slope from the upstream section to the downstream section. Since little general scour was observed at the upstream and downstream sections the mean elevation of the unscoured portion of the WSPRO bridge section will be used as the contraction scour reference surface, elevation 981.5 ft.

The contracted section on 4/5/97 was measured under the bridge from data collected by an acoustic Doppler current profiler. The depths represent a weighted average of the four beam depths. Because a weighted-average was used it is possible that the local abutment scour was not detected. The maximum lowering of the stream bed was actually 7.5 ft, however, when the entire bed below the bridge was averaged the depth of contraction scour was only 3.1 ft. The hydraulic data presented for measurement number 1 were collected with the ADCP. The ADCP data showed many missing ensembles that were estimated in the final processing. There was not clear delineation of the channel banks in the approach section, creating a degree of uncertainty in the approach discharge. Overall it is expected that the approach discharge is +/- 20% and the total discharge is +/- 10%.

Measurements number 2 was made during a discharge measurement along the upstream face of the bridge. The depths were measured with a sounding weight. Measurements 3 and 4 were made using an echo sounded mounted on a knee-board. The board was floated from upstream to downstream under the bridge. The measurements reflect the depths at the upstream or downstream face of the bridge.

The cross sections measured on 4/9/97 all showed a similar pattern with abutment scour holes on each side and a sharp mound in between the scour holes but skewed towards the left bank. It appears that the abutment scour holes may have overlapped. The highest elevation in the center of the cross section was subtracted from the reference surface to obtain the depth of contraction scour. No data in the approach section was collected on 4/9/97.

Measurement No. 2

See comments on measurement no. 1.

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Measurement No. 3

See comments on measurement no. 1.

Measurement No. 4

See comments on measurement no. 1.

Stage and Discharge Data

Pe	Peak Discharge		Flow		Peak Stage			Stage	Water		Return				
year	mo	dу	hr	mi	(cfs)	Qacc	year	mo	dу	hr	mi	(ft)	Temp	(C)	Period(yr)
							1997	4	9			1021.9)		
							1997	4	5			1019.4	ļ		

Hydrograph

Supporting Files

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us12pdt-REV.xls - contains the following data:
Summary - Summary of basic site and scour data
Hydrograph - Hydrograph from nearest USGS gaging station
X-Sec - cross section data
The following photos were scanned from a black and white copy of the
bridge scour evaluation report completed by BRW:
pdt12-scrrpt-ds-channel.jpg
pdt12-scrrpt-abuts.jpg
pdt12-scrrpt-bridge.jpg
pdt12-scrrpt-nwcorner-bridge.jpg
pdt12-scrrpt-us-channel.jpg
pdt12-scrrpt-us-dam.jpg
pdt12-brgpln-siteplan.jpg is a site plan scanned from the bridge plans
provided by MnDOT.
pdt12-flood-us-bridge.jpg is a photo taken during the flood, from the
right bank looking across the face of the bridge to the left
floodplain. Note the slump in the foreground.
pdt12-flowfield.jpg - sketch of flow field observed on 4-9-97
pdt12-rwingwall - photo of data collection along the right upstream
wingwall. Note the slump in the embankment.
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